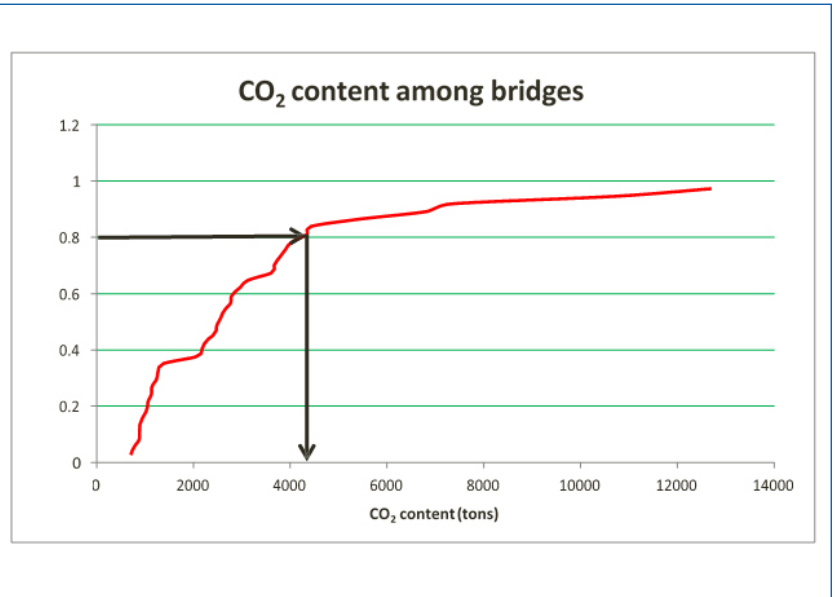


MOUNTAIN-PLAINS CONSORTIUM

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Quantifying Sustainability Metrics for Trunk Line Bridges in the Mountain Plains Region



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**QUANTIFYING SUSTAINABILITY METRICS FOR TRUNK LINE
BRIDGES IN THE MOUNTAIN PLAINS REGION**

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September 2018

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EXECUTIVE SUMMARY

Millions of cubic yards of concrete and steel used to support the U.S infrastructure may result in a significant negative impact on the environment. CO₂ released by construction processes, as well as material production, is taking a substantial toll on the environment. This study seeks to develop a system to rank bridges based on their CO₂ emissions. First, in order to accomplish this objective, rating systems for buildings around the world were analyzed for common attributes applicable to bridges. Second, a sample of bridges from the state of Colorado was selected and analyzed for sustainability by considering only their primary materials. This sample served as the first step in developing a sustainability rating system for bridges in Colorado, where production and transport of concrete and steel were considered in this analysis. This rating system can be further developed to include CO₂ emissions from construction processes, demolition and disposal and other factors that contribute to sustainability, but its current version is intended only to serve as an example of an approach to develop a ranking system.

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1. INTRODUCTION

One of the most important aspects of modern civil engineering design is the inclusion of design practices where environmental impacts are major concerns. There is growing concern about the planet's long-term future resources, population growth, and environmental degradation. According to the U.S Environmental Protection Agency (EPA), electricity production generates the largest share (31%) of greenhouse gas (GHG) emissions, with the second largest producer of GHG being transportation (27%) (EPA, 2013). One of the other indirect contributors to the rise in global temperatures and climate change is the increasing population. According to the World Bank (The World Bank, 2013), the world has experienced an unprecedented increase in population growth, and it is projected to increase to at least eight billion by the end of 2050. With the population on the rise, the construction industry is experiencing a higher demand for additions to the physical infrastructure than ever before. This increasing demand is well understood, as seen by the increase in civil engineering graduates throughout America. According to Yoder (2011), using data from the American Society for Engineering Education (ASEE), during the 2011-2012 academic year there were 12,309 civil engineering undergraduate students in the country (Yoder, 2011). That number continues to rise.

Even though an increase in the number of civil engineers and civil engineering projects may be seen as a good indicator of economic production, it also comes with a cost. Due to a rise in commercial and residential buildings, the U.S Green Building Council (USGBC) states that buildings are one of the heaviest consumers of natural resources, and account for a significant portion of the GHG gas emissions that affect climate change. In the United States, buildings account for 38% of all CO₂ emissions and 73% of electricity consumption (USGBC, 2015)

Despite such conditions, the civil engineering industry soon recognized the problem, and is now working toward building and designing energy efficient buildings that have a minimal impact on the environment. The USGBC reported (McGraw-Hill, 2013) that green building construction around the world is accelerating and becoming viewed as a long-term opportunity. A majority of the architects, engineers and contractors, owners, and consultants participating in the study anticipate that more than 60% of their work will be green by 2015, up from 28% of firms polled in 2012.

However, commercial and residential buildings are only a portion of the physical infrastructure. Infrastructure systems, especially transportation systems, within civil engineering have not yet received adequate attention when it comes to sustainable design/construction practices.

According to a study performed by Korkmaz (2012), transportation is a vital part of the economy but also a significant source of GHG emission. It involves several construction activities, which directly or indirectly release GHG, water, and land pollutants (Korkmaz, 2012).

Before any sustainability standards for infrastructure systems can be enforced, there first must be a methodology to quantify it. One such example of considering criteria for sustainability in infrastructure construction projects is a study performed in the United Kingdom (UK) entitled, "Quantification of Sustainability Principles in Bridge Projects" (Spencer et al., 2012). The study provided information on key attributes to consider while developing a bridge project metric that relates to the economy, society, environment, resources, and climate change. The UK's Building Research Establishment Environmental Assessment Methodology (BREEAM) has been described as the world's foremost environmental assessment method and rating system for buildings (Fowler and Rauch, 2006). BREEAM suggests that while proposing any local requirements for sustainable buildings, planning authorities must be able to demonstrate with clear robust evidence the circumstances that warrant these requirements and focus on local or site-specific opportunities and constraints (BREEAM, 2012).

Based on BREEAM's recommendation and the study by Spencer et al., a basic metric is used to characterize the sustainability of a bridge in Colorado, and which can be further developed to include other aspects of bridge sustainability. Looking solely at bridge superstructures described later in this thesis, the metric is applied to a small but representative group of bridges in Colorado to provide information on bridge sustainability.

2. METHODOLOGY

Worldwide, there are hundreds of building evaluation tools that focus on different areas of sustainable development and are designed for different types of projects, such as life cycle assessment, life cycle costing, energy systems design, performance evaluation, and productivity analysis (Fowler and Rauch, 2006). Many of the systems developed in different countries around the world were created by modifying a single system or integrating multiple systems (Fowler and Rauch, 2006). Examples of such single systems are LEED (Leadership in Energy & Environmental Design), BREEAM Green Building Tool, Green Globe US, and Comprehensive Assessment System for Building Environmental Efficiency (CASBEE) (Fowler and Rauch, 2006).

The study presented herein is divided into two sections. First, it looks at the derivation of standards to quantify sustainability of bridges from different green building rating systems around the world. Second, one of the chosen criteria for sustainability of bridges is quantified for a select number of bridges from the Colorado bridge inventory.

Most of the rating systems around the world share a common goal of creating sustainable structures; therefore, a number of rating systems available in several different countries around the world were analyzed. Rating systems analyzed were from the United States, United Kingdom, Australia, Japan, China, France, Germany, India, Malaysia, South Africa, Austria, and Canada. Each country had one or more rating systems, and all those that could be identified were analyzed based on the availability of credible data. Different rating systems had different criteria for awarding a building with a status of sustainability. Only the criteria that were repeated two or more times were considered in the analysis in the present study.

Each system analyzed awarded its certification to a structure based on a pre-established scale to determine sustainability. Furthermore, it was common for a rating system to have subdivisions in all of its major criteria, as is done by LEED-USA (USGBC, 2009). For achieving and/or including certain features from its predetermined list, it awards points to the building. Based on the number of points achieved by a building, it is awarded the platinum, gold, silver, or certified certification with platinum being the highest achievable certification and certified being the lowest achievable (LEED, n.d.). In line with this concept, sustainability ratings for bridges are believed to be positive and can also be awarded a “rank,” which makes it easier to understand the bridge’s level of sustainability.

All rating systems analyzed herein were used only for residential and commercial buildings. Even though some of the rating systems extended to quantify sustainability for renovating structures, only new building construction was taken into consideration for this study. Initially, details of the criteria used by each rating system to award sustainability were procured and then each was classified into general categories of similar nature. Note that some rating systems had criteria that were not used in other rating systems, so were disregarded with the assumption that they only pertained to the region where that rating system is operated.

The criterion was then identified and subdivided to develop a better understanding of the conditions met to fulfill the criteria for each rating system. An example of this process is the checklist formulated by LEED-USA to fulfill the criteria related to having sustainability in materials and resources while constructing a new building. Table 2.1 shows the checklist of attributes that should be fulfilled for achieving credits for a LEED sustainability rating (USGBC, 2009).

Table 2.1 Attributes for material sustainability for LEED

Credit #	Description	Required points
Prerequisite 1	Storage and Collection of Recyclables	Required
Credit 1.1	Building Reuse-Maintain Existing Wall, Floors and Roof	1-3
Credit 1.2	Building Reuse-Maintain Existing Interior Nonstructural Elements	1
Credit 2	Construction Waste Management	1-2
Credit 3	Materials Reuse	1-2
Credit 4	Recycled Content	1-2
Credit 5	Regional Materials	1-2
Credit 6	Rapidly Renewable Materials	1
Credit 7	Certified Wood	1
	Total Possible Points	14

The approach presented in this study attempts to parallel building sustainability for basic metrics used for bridge construction. By figuring out the criteria that can be applied to bridges, it is then possible to award points/checks such that they are fulfilled during or after the construction of a new bridge. The flowchart presented in Figure 2.1 summarizes the steps described above.

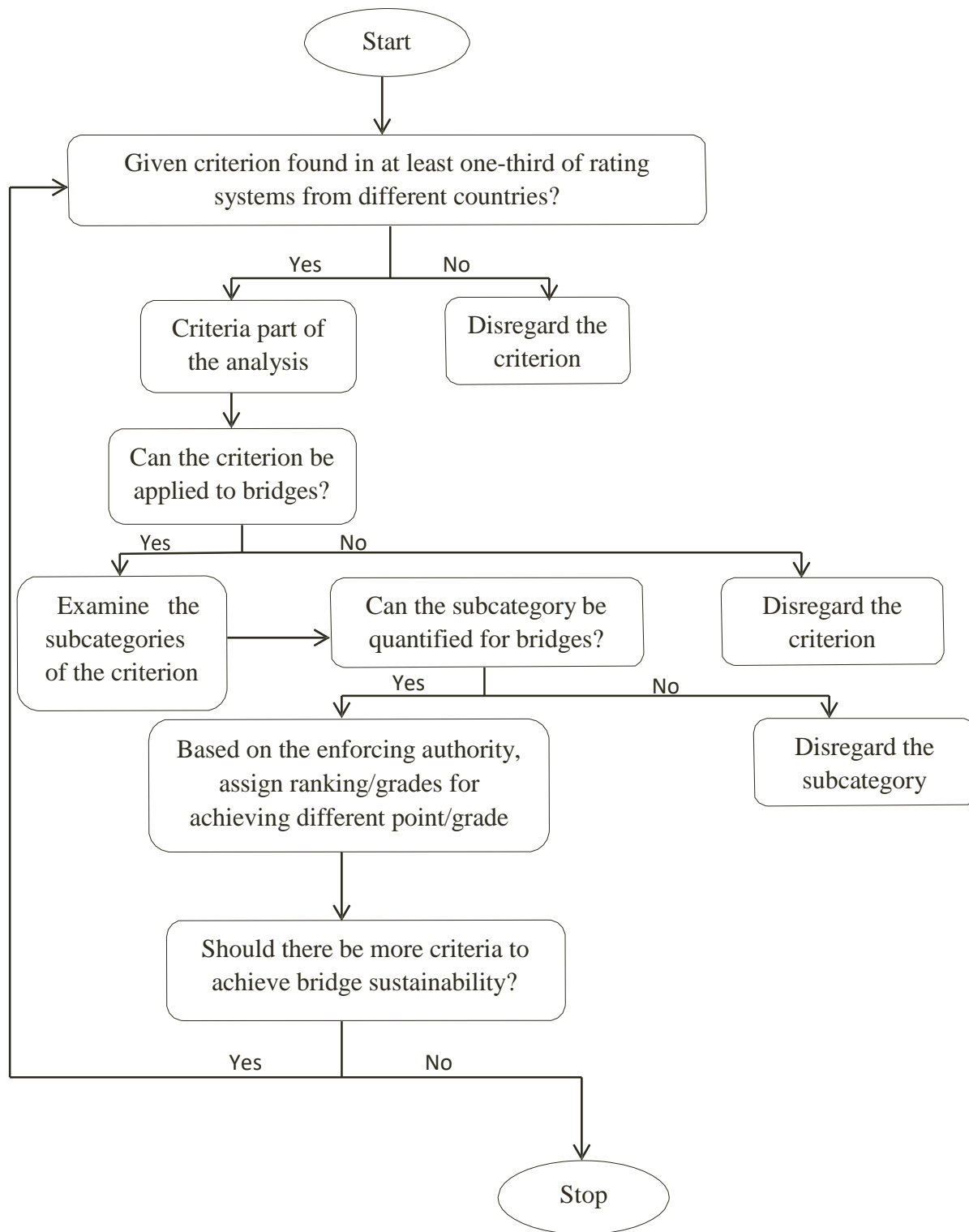


Figure 2.1 Flowchart describing Stage 1 of the study

After following the process mentioned in Stage 1, the criteria with which the sustainability of a bridge can be quantified was identified as five general categories. These categories are sustainability in materials used for construction, energy (electricity and crude oil products) used during the construction and operation of the bridge, selection of site for bridge construction, air quality, and water use for construction and operation of the bridge. Finding such general categories that can be further broken down into sub-categories constitutes the end of Stage 1.

For Stage 2, one criterion, sustainability in materials, was selected to serve as a surrogate for sustainability in general. Only one of the criteria was considered because including all the aspects of sustainability in bridges would be beyond the information available at this stage. Also, this criterion was selected because of the forbearance in acquiring data as well as its direct link to sustainability of a structure. But before starting data collection and analysis, an important assumption made in this study should be described. Each bridge's superstructure carbon footprint was assumed to be representative of its sustainability, and the sample size used was assumed to be representative of Colorado's bridge population. The construction and maintenance of the bridge were not considered in the carbon footprint calculation since it can be built into the rating system with the framework provided in this study.

The carbon footprint was chosen as sustainability metric due to its adverse effects on the environment. Climate scientists have observed that carbon dioxide (CO₂) concentrations in the atmosphere have been increasing significantly over the past century, compared with the rather steady level of the pre-industrial era (about 280 parts per million in volume, or ppmv). The 2013 concentration of CO₂ (396 ppmv) was about 40% higher than in the mid-1800s, with an average growth of 2 ppmv/year in the last 10 years. Significant increases have also occurred in levels of methane (CH₄) and nitrous oxide (N₂O) (2012 CO₂ Emissions Overview, n.d.). Furthermore, the EPA states that cement production is a key source of CO₂ emission, due in part to its significant reliance on coal and petroleum coke to fuel the kilns for clinker production. Globally, CO₂ emissions from cement production were estimated at 829 MMTCO₂ in 2000, or approximately 3.4% of global CO₂ emissions from fossil fuel combustion and cement production (Hanley et al., n.d.). Similarly, the steel industry also generates a significant amount of CO₂ and other greenhouse GHGs. The GHG emissions in steelmaking are generated as one of the following: (1) process emissions, in which raw materials and combustion both may contribute to CO₂ emissions; (2) emissions from combustion sources alone; and (3) indirect emissions from consumption of electricity, primarily in electric arc furnaces (EAFs) and in finishing operations such as rolling mills at both integrated and EAF plants. For EAF steelmaking, the primary sources of GHG emissions include indirect emissions from electricity usage (50%), combustion of natural gas in miscellaneous combustion units (40%) and steel production in the EAF (10%) (Jones, 2012). Such data related to cement and steel production suggest that the main CO₂ contributions from such industries are due to their energy usage. According to the EPA, the combustion of fossil fuels to generate electricity is the largest single source of CO₂ emissions in the nation, accounting for about 37% of total U.S. CO₂ emissions and 31% of total U.S. GHG emissions in 2013 (EPA, October 2015). For these reasons, CO₂ content of a bridge was assumed to be the measure of its sustainability.

Initially, bridges fulfilling certain criteria were selected from the complete list of bridges archived by the Colorado Department of Transportation (CDOT). The list of bridges selected from the CDOT inventory is shown in Appendix A. Since the material aspect of sustainability was to be quantified, only the essential bridge building materials, such as concrete and steel, were considered, and each bridge's CO₂ emission was assumed to be the CO₂ footprint of the entire bridge. Estimating the CO₂ content of each bridge was based on published data on carbon content as well as existing approaches available in the literature. After quantifying the CO₂ content, it was

normalized using several criteria, such as the number of lanes, deck area, and unit width, and CO₂ content charts were developed.

Similar to Stage 2, other criteria can be analyzed and quantified accordingly. After quantifying the entire criterion, it can be normalized to provide a rating scale to which ranking/grade can be assigned and hence build a full-scale rating system for bridges. Figure 2.2 shows a flowchart summarizing the procedure described above.

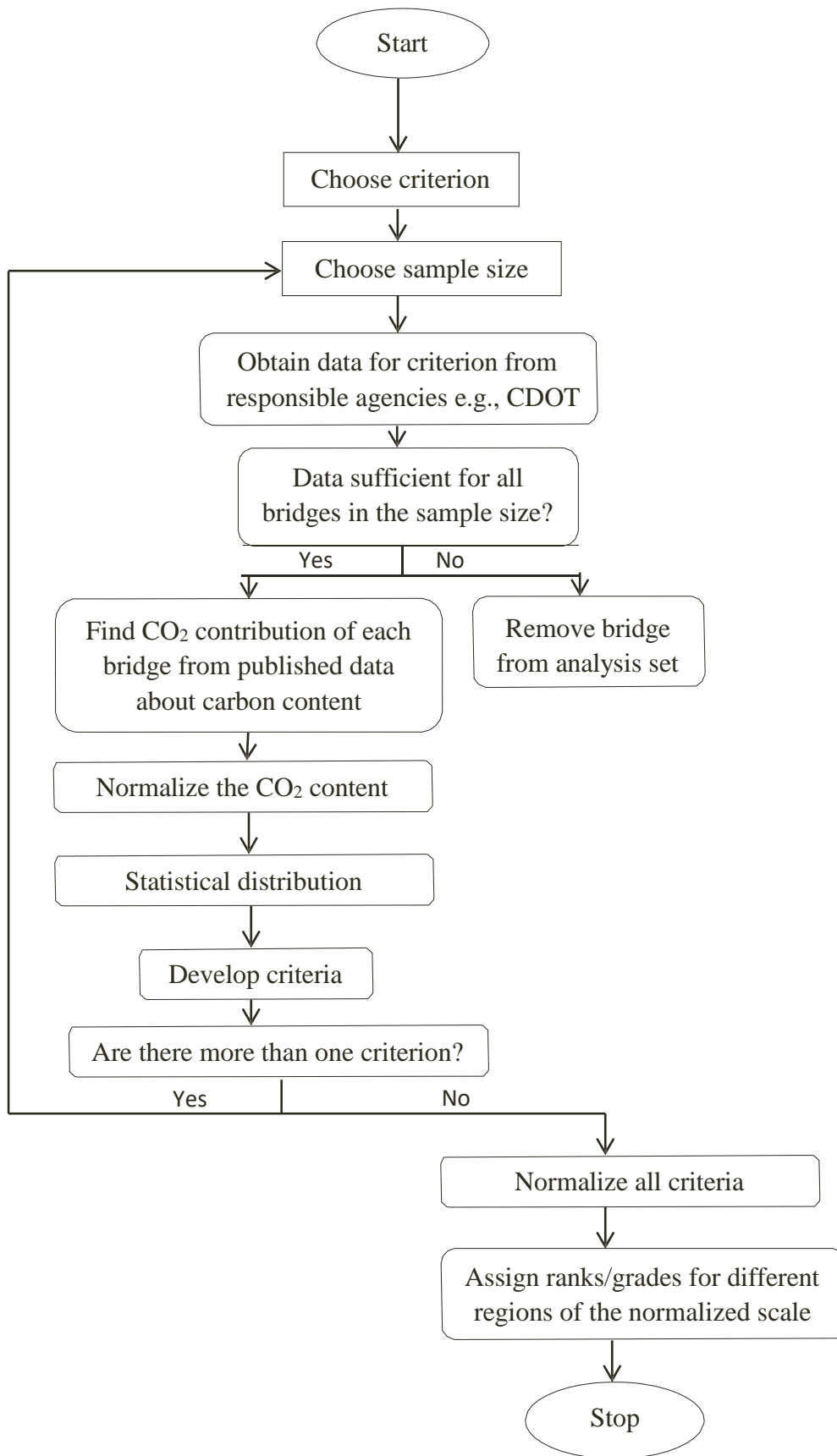


Figure 2.2 Flowchart describing Stage 2 of the study

3. ILLUSTRATIVE EXAMPLE

In this section, an example of the study cited in Section 2 is presented for Colorado, which is felt to be representative of the Mountain Plains Region. For this example, due to the vast number of bridges in service in Colorado, 36 randomly selected bridges were included in the procedure. All of the bridges selected were constructed after 1990 and had a length of at least 200 feet.

Moreover, the bridges chosen had their main structural element to be either prestressed concrete or steel.

3.1 Assumptions

For each bridge chosen, Table 3.1 shows the assumptions made before quantifying the CO₂ footprint.

Table 3.1 Analysis Assumptions

	Assumption	Variable definition
1	CO ₂ content is a representation of the bridge's sustainability	-
2	Area of the bridge	Deck Length (ft) * Deck width (ft)
3	Average days to construct	90 days
4	Service life	75 years
5	Main material	Steel or Concrete
6	Distance travelled by concrete before pouring via road (to and fro)	100 miles
7	Distance travelled by steel before erection via road	560 miles (Plymouth, UT to Denver, CO)
8	Average strength concrete	5 ksi
10	Steel fabrication level	CO ₂ emissions related to steel fabrication require data on structural design: complicated designs may require more detailing and fabrication. Therefore, the fabrication level is assumed to be average with the CO ₂ emissions related to it assumed to be 0.020 Kg
11	Wind loading	>58 m/s
12	Seismic loading	0.38g to 0.95g

3.2 Procedure

After making the assumptions for the select bridges, their structural drawings were obtained from the Colorado Department of Transportation (CDOT). From the obtained bridge plans, the quantity of concrete and steel used in the construction of the superstructure was quantified for each bridge. This was followed by finding the quantity of structural steel, concrete, and rebar per square foot area of the bridge deck to be included in the Environmental Analysis (EA) tool program created by Skidmore, Owings & Merrill LLP (SOM). The reason for choosing the EA tool was the credibility in their calculation of CO₂ content. All the data regarding the CO₂ in various attributes were derived from various government organizations and universities around the globe. Some of those sources include the National Renewable Energy Laboratory (NREL), University of Bath, Inventory of Carbon and Energy, Portland Cement Association (PCA), California Energy Commission, Carnegie Mellon University, and South Coast Air Management District.

The EA Tool program measures the equivalent carbon dioxide (CO₂e) emission for a structure. In doing so it accounts for all the GHGs besides CO₂ that contribute toward a 100-year global warming potential (GWP) for the structure. To sum up the contribution from each of these gases to the total GWP, factors are assigned to each gas based on molecular weight using CO₂ as the benchmark. These factors are summarized in Table 3.2.

Table 3.2 GWP factor for each GHG gas (EA Tool, 2013)

GHG	GWP Factor (equivalent CO₂)
Carbon dioxide	1
Dinitrogen monoxide	310
Methane	21
Methane, HCC-30	9
Nitrogen oxides	Negligible
Nonmethane VOCs	Negligible
Carbon Monoxide	Negligible

After determining the equivalent CO₂, the developers then applied it to the equivalent CO₂ contribution by materials production. All the data shown in Tables 3.3 to 3.7 are derived from the EA Tool user manual. Since procuring building materials utilize energy sources for production, transportation, and installation, the equivalent CO₂ emission for using energy is quantified as shown in Table 3.3.

Table 3.3 CO₂ equivalent for 1 MJ production of energy (EA Tool, 2013)

For 1.0 MJ of energy	Emission	Unit	Factor	Emission	Unit
Embodied carbon dioxide	0.194061	kg	1	0.194061	kg CO ₂ e
Other GHGs:					
Dinitrogen monoxide	0.000001	kg	310	0.000169	kg CO ₂ e
Methane	0.000002	kg	21	0.000041	kg CO ₂ e
Methane, HCC-30	0	kg	9	0	kg CO ₂ e
Nitrogen oxides	0.000473	kg	0	0	kg CO ₂ e
Nonmethane VOCs	0.000005	kg	0	0	kg CO ₂ e
Carbon monoxide	0.000038	kg	0	0	kg CO ₂ e
Total Equivalent Embodied Carbon dioxide:				0.194272	kg CO₂e

Similarly, the equivalent amounts of CO₂ emission for production of all steel components and its fabrication as well as concrete products are shown below. Tables 3.4, 3.5, and 3.6 take into account the CO₂ contribution of energy usage as well as raw materials used in the production.

Table 3.4 Equivalent CO₂ content for 1 kg production of steel components (EA Tool, 2013)

For 1.0 kg of steel	Emission	Unit	Factor	Emission	Unit
Embodied carbon dioxide	2.27118	kg	1	2.27118	kg CO ₂ e
Other GHG's:					
Dinitrogen monoxide	3E-06	kg	310	0.00081	kg CO ₂ e
Methane	0.00113	kg	21	0.02371	kg CO ₂ e
Methane, HCC-30	0	kg	9	0	kg CO ₂ e
Nitrogen oxides	0.00282	kg	0	0	kg CO ₂ e
Nonmethane VOCs	0.00107	kg	0	0	kg CO ₂ e
Carbon monoxide	0.02491	kg	0	0	kg CO ₂ e
Total Equivalent Embodied Carbon dioxide:				2.2957	kg CO ₂ e

Since uniform data for the fabrication process for steel shapes are not readily available, the following quantities in Table 3.5 (SOM, 2013) are assumed for all the bridges. Fabrication for other steel components, such as nuts, bolts, and rebar, are not considered since the majority of them are manufactured without the need for any further fabrication.

Table 3.5 Equivalent CO₂ emission for rolled shapes fabrication (EA Tool, 2013)

Material (1 kg)	Low-level Type Fabrication	Average-level Type Fabrication	High-level Type Fabrication
Structural Steel – Rolled Shapes	0.010 kg CO ₂ e	0.020 kg CO ₂ e	0.030 kg CO ₂ e

Table 3.6 shows the equivalent CO₂ contribution by different concrete strength types. Further details required for calculating the equivalent CO₂ shown in Table 3.6 can be found in Appendix B. It provides information on the equivalent CO₂ emission due to the production of cement and transportation of other cementitious materials to the concrete mix plant. Furthermore, details on equivalent CO₂ emission for different concrete mixes have also been included in Appendix B.

Table 3.6 Equivalent CO₂ emission for varying concrete strengths (EA Tool, 2013)

Strength Type (1 kg)	Mix Ratio by Weight – Cement: Sand: Coarse Agg.	kg CO ₂ (equivalent)
Low-strength	1:02:04	0.092
Average-strength	01:05.5	0.128
High-strength	1:01:02	0.19

After calculating the CO₂ equivalent contribution by the main materials used in the bridge, the equivalent CO₂ contribution by transportation of those materials are shown in Table 3.7.

Table 3.7 Equivalent CO₂ emission to a heavy truck (EA Tool, 2013)

For 1.0 km of transport by heavy-duty truck	Emission	Unit	Factor	Emission	Unit
Embodied carbon dioxide	1.186926	kg	1	1.187	kg CO ₂ e
Other GHGs:					
Dinitrogen monoxide	0	kg	310	0	kg CO ₂ e
Methane	0.00004	kg	21	0.000841	kg CO ₂ e
Methane, HCC-30	0	kg	9	0	kg CO ₂ e
Nitrogen oxides	0.010773	kg	0	0	kg CO ₂ e
Nonmethane VOCs	0	kg	0	0	kg CO ₂ e
Carbon monoxide	0.003369	kg	0	0	kg CO ₂ e
Total Equivalent Embodied Carbon dioxide:				1.187767	kg CO ₂ e

With all the equivalent CO₂ content calculated for production and transportation of materials, SOM then incorporated it into the EA Tool. But before running the software, the material consumption in each bridge was quantified in accordance with the EA Tool program. Figures 3.1 to 3.7 show the inputs required by the program from each bridge in the analysis for this thesis. As an example, the data from a prestressed bridge (C-20-AS) are included as the data input.

C-20-AS		Project Title													
<input checked="" type="radio"/> Imperial (ft, lbf) <input type="radio"/> Metric (m, kN)		Project Units													
<table border="1"> <thead> <tr> <th></th> <th>Superstructure</th> <th>Substructure</th> </tr> </thead> <tbody> <tr> <td>Number of stories</td> <td>1</td> <td>0</td> </tr> <tr> <td>Area per floor, sq ft</td> <td><input checked="" type="radio"/> 19180.338²</td> <td><input checked="" type="radio"/> 0</td> </tr> <tr> <td>Total floor area, sq ft</td> <td><input type="radio"/> 19180.338²</td> <td><input type="radio"/> 0</td> </tr> </tbody> </table>			Superstructure	Substructure	Number of stories	1	0	Area per floor, sq ft	<input checked="" type="radio"/> 19180.338 ²	<input checked="" type="radio"/> 0	Total floor area, sq ft	<input type="radio"/> 19180.338 ²	<input type="radio"/> 0	Building Size (Foundations Included)	
	Superstructure	Substructure													
Number of stories	1	0													
Area per floor, sq ft	<input checked="" type="radio"/> 19180.338 ²	<input checked="" type="radio"/> 0													
Total floor area, sq ft	<input type="radio"/> 19180.338 ²	<input type="radio"/> 0													
Concrete		Main Structural Material													
0 Average days per story		Construction Time													
75 years		Service Life													
Moderate		Wind Loading													
		<input checked="" type="radio"/> Approximate <input type="radio"/> Exact													
Moderate		Seismic Loading													
		<input checked="" type="radio"/> Approximate <input type="radio"/> Exact <input type="radio"/> Look up by zip code													
<input checked="" type="radio"/> Conventional System <input type="radio"/> Enhanced System		Seismic Force Resisting System													
Life Safe		<input checked="" type="radio"/> Empirically based <input type="radio"/> HAZUS-based													
Performance Level at Design Basis Earthquake															

Figure 3.1 Data input section for preliminary data about bridge (EA Tool, 2013)

All the seismic and wind loading is done in accordance with International Building Codes (IBC) (2006). For moderate wind loading, the IBC defines as forces exerted by winds of speed 45 to 58 m/s; and for moderate seismic loading, the IBC has a value of 0.38g to 0.95g for the spectral response acceleration (S_s). It can be noted that the number of days for construction is stated to be zero, which is because this thesis is limited to examination of the equivalent CO₂ contribution solely from the material production and transportation in the analyses.

Figure 3.2 shows the input section for materials used in the construction of the bridge. It should be noted that, in order to include the quantities of steel and concrete, it had to be separated into units of pounds of structural steel per square foot, cubic feet of concrete per square foot, and pounds of rebar per square foot. Such data were collected from bridge plans provided by CDOT. Only concrete and steel present in the superstructure (slabs and girders) of the bridge were considered in this section of the input.


<input type="text" value="8.002"/>	Steel, psf	<input type="text" value="Average"/>	Steel Fabrication Level
<input type="text" value="1.85885273"/>	Concrete, cf/sf		
<input type="text" value="0"/>	Percentage flyash, %	<input type="text" value="0"/>	Percentage slag, %
<input type="text" value="0"/>	Percent low-strength concrete		
<input type="text" value="0"/>	Percent medium-strength concrete		
<input type="text" value="100"/>	Percent high-strength concrete		
<input type="text" value="8.289008985"/>	Rebar, psf		
<input type="text" value="0"/>	Metal deck, psf		
<input type="text" value="0"/>	Wood Dim. Softwood Lumber, cf/sf		
<input type="text" value="0"/>	Wood Panels, cf/sf		
<input type="text" value="100"/>	Percentage plywood, %	<input type="text" value="0"/>	Percentage OSB, %
<input type="text" value="0"/>	Wood Glulam, cf/sf		
<input type="text" value="0"/>	Wood Timber Trusses, cf/sf		
<input type="text" value="0"/>	CMU, No. of blocks per sf	<input type="text" value="0"/>	CMU, cf/sf
<input type="text" value="0"/>	Percentage flyash, %	<input type="text" value="0"/>	Percentage slag, %
<input type="text" value="0"/>	Cold-Formed Steel, including Fasteners, psf		

Figure 3.2 Data input section for materials used in superstructure (EA Tool, 2013)

Figure 3.3 shows the input section for material transportation data. The distance concrete and steel travels before reaching the plant/work site is entered as per the assumptions in Section 3.1. In considering the truck’s return trip to the plant, note that the distance concrete travels is twice as much as mentioned in the assumption. Steel delivery is not the same case since the trucks delivering steel typically get another transport job assigned for their return journey (EA Tool, 2013).

Steel Distances, mi		Wood Distances, mi	
<input type="text"/>	Ocean freighter, diesel	<input type="text" value="0"/>	Ocean freighter, diesel
<input type="text"/>	Barge, diesel	<input type="text"/>	Barge, diesel
<input type="text"/>	Train freighter, diesel	<input type="text"/>	Train freighter, diesel
<input type="text" value="562"/>	Truck, diesel	<input type="text"/>	Truck, diesel
Concrete Distances, mi		CMU Distances, mi	
<input type="text" value="0"/>	Ocean freighter, diesel	<input type="text" value="0"/>	Ocean freighter, diesel
<input type="text" value="0"/>	Barge, diesel	<input type="text" value="0"/>	Barge, diesel
<input type="text" value="0"/>	Train freighter, diesel	<input type="text" value="0"/>	Train freighter, diesel
<input type="text" value="100"/>	Truck, diesel	<input type="text"/>	Truck, diesel
Rebar Distances, mi		Cold-Formed Steel Distances, mi	
<input type="text" value="0"/>	Ocean freighter, diesel	<input type="text" value="0"/>	Ocean freighter, diesel
<input type="text"/>	Barge, diesel	<input type="text" value="0"/>	Barge, diesel
<input type="text"/>	Train freighter, diesel	<input type="text"/>	Train freighter, diesel
<input type="text" value="562"/>	Truck, diesel	<input type="text"/>	Truck, diesel
Metal Deck Distances, mi			
<input type="text"/>	Ocean freighter, diesel		
<input type="text"/>	Barge, diesel		
<input type="text"/>	Train freighter, diesel		
<input type="text"/>	Truck, diesel		

Figure 3.3 Data input section for material transportation (EA Tool, 2013)

Figure 3.4 and 3.5 indicate there is no consideration of CO₂ emission from equipment to support the construction of the bridge. Since the study focuses on prestressed and steel bridges, the usage of such equipment varies widely for construction of each bridge type, and including them in the analyses can skew the results intended to obtain the CO₂ emission solely from material usage. Applying the technique described in this thesis to a specific bridge would require a relatively easy to obtain list of equipment and durations from the contractor.

<input type="text" value="0"/> Percentage of grid power from renewable sources		
<u>Process</u>	<u>No. of units</u>	<u>Time on site /in use (days)</u>
<u>Diesel</u>		
Regular crane operation	<input type="text" value="0"/>	<input type="text" value="0"/>
Tower crane operation	<input type="text" value="0"/>	<input type="text" value="0"/>
Elevator operation	<input type="text" value="0"/>	<input type="text" value="0"/>
Lifting / moving w/o crane	<input type="text" value="0"/>	<input type="text" value="0"/>
Welding	<input type="text" value="0"/>	<input type="text" value="0"/>
Mixing concrete	<input type="text" value="0"/>	<input type="text" value="0"/>
Pumping concrete	<input type="text" value="0"/>	<input type="text" value="0"/>
Bucketing concrete	<input type="text" value="0"/>	<input type="text" value="0"/>
Placing concrete	<input type="text" value="0"/>	<input type="text" value="0"/>
Fireproofing	<input type="text" value="0"/>	<input type="text" value="0"/>
Miscellaneous watering	<input type="text" value="0"/>	<input type="text" value="0"/>
Miscellaneous blowing	<input type="text" value="0"/>	<input type="text" value="0"/>
Miscellaneous powering	<input type="text" value="0"/>	<input type="text" value="0"/>
<u>Electric</u>		

Figure 3.4 Input box for diesel construction equipment (EA Tool, 2013)

Electric		
Bolting	<input type="text" value="0"/>	<input type="text" value="0"/>
Bending	<input type="text" value="0"/>	<input type="text" value="0"/>
Jacking / stressing	<input type="text" value="0"/>	<input type="text" value="0"/>
Vibrating	<input type="text" value="0"/>	<input type="text" value="0"/>
Lighting	<input type="text" value="0"/>	<input type="text" value="0"/>
Heating	<input type="text" value="0"/>	<input type="text" value="0"/>
Woodcutting	<input type="text" value="0"/>	<input type="text" value="0"/>
Forming	<input type="text" value="00"/>	<input type="text" value="0"/>

Figure 3.5 Input box for electric construction equipment (EA Tool, 2013)

Figure 3.6 shows the input section to account for probabilistic damage of the structure. This section is left as zero because the bridge is assumed to be functioning with no damage and without the need for any demolition or significant rehabilitation over a realistic analysis time frame.

<input type="text" value="0"/>	FullyOperational
<input type="text" value="0"/>	Operational
<input type="text" value="0"/>	Life Safe
<input type="text" value="0"/>	Near Collapse
<input type="text" value="0"/>	Demo Steel, kg CO2 / sq ft
<input type="text" value="0"/>	Demo Conc. kg CO2 / sq ft
<input type="text" value="0"/>	Demo Wood, kg CO2 / sq ft
<input type="text" value="0"/>	Demo CMU, kg CO2 / sq ft
<input type="text" value="0"/>	Demo Cold-Formed Steel, kg CO2 / sq ft

Figure 3.6 Input box for probabilistic damage (EA Tool, 2013)

With the materials in the superstructure quantified as shown in Figure 3.2, materials used in the foundation construction is quantified as shown in Figure 3.7. Data in Figure 3.7 represent the amount of concrete and steel used in the piers and other support features of a bridge. All the data presented in this example for the bridge C-20-AS are also presented in Appendix C for further reference.

Superstructure
Substructure
Foundation

Material Quantities
Construction

System-generated default material quantities criteria

User-input material quantities criteria

By Quantities

Steel, psf Average Steel Fabrication Level

Concrete, cf/sf

Percentage flyash, % Percentage slag, %

Percent low-strength concrete

Percent medium-strength concrete ?

Percent high-strength concrete

Rebar, psf

By Types

R/C Bored Pili Pile Type

Diameter of piles, ft

Number of piles

Length of Piles, ft

Pile Capacity, tons

Thickness of Mat Foundation Slab, ft

Figure 3.7 Data input section for materials in the foundation (EA Tool, 2013)

After applying the EA program with the inputs mentioned above, as well the above mentioned assumptions, it returned the amount of CO₂ equivalents in tons produced during the production and transportation of materials for the bridge, as shown in Figure 3.8. This procedure is repeated for all bridges within the sample size to get the total amount of CO₂ produced in the materials used for construction of each bridge.

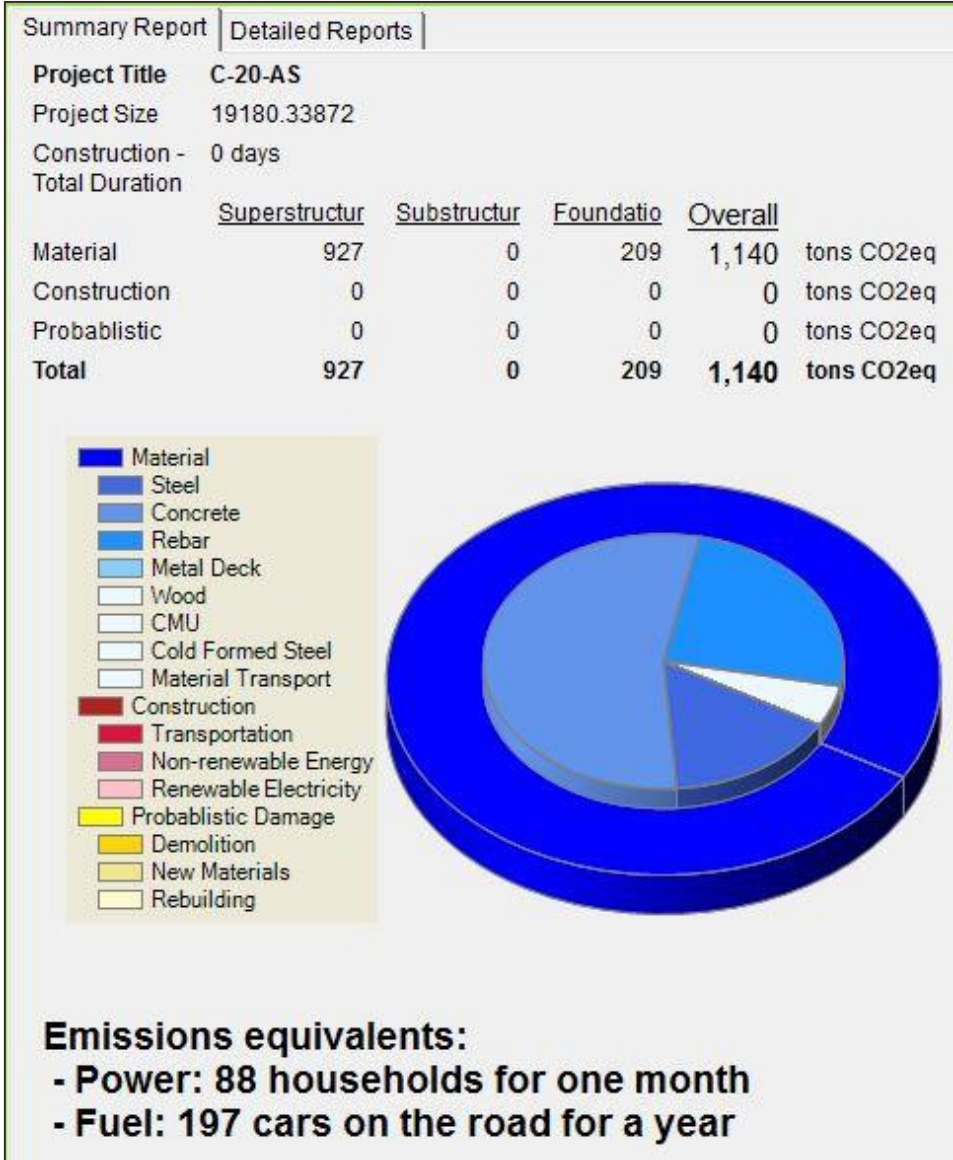


Figure 3.8 Analysis output for bridge C-20-AS (EA Tool, 2013)

Note from Figure 3.8 that the amount of equivalent CO₂ returned by the analyses is only for the production of concrete and steel and its transportation to the construction site. Doing so gives a good representation of GHG emissions solely from the use of such materials.

For analyses of alternatives for a specific bridge, the differences in CO₂ emissions for concrete versus steel construction techniques could be determined from contractor equipment and duration estimates. Additionally, differences in CO₂ emissions contributed by long-life maintenance and of end-of-life demolition could be estimated as well. Doing so for a sample of 36 bridges exceeded the scope of this research.

After deriving the CO₂ consumption of each bridge before it began its service life, it is then normalized per square feet area of the deck, per lane, as well as per unit width of each bridge using the Weibull plotting position. Results obtained are shown in the next section. A plot of total CO₂ consumption for all bridges in the sample size was also obtained.

3.3 Results

With the goal of developing a ranking system for sustainability of trunk line bridges, the bridges in the sample size obtained were analyzed for their CO₂ contribution. The main assumption being the CO₂ contribution from bridges is an indicator of their sustainability along with other assumptions made in Section 3.1; and by using the analysis method described in Section 3.2, the bridges in the sample size were analyzed. After the analysis of each bridge, its CO₂ consumption was tabulated along with the CO₂ data from other bridges. The results were rank ordered to develop an empirical cumulative distribution function, as shown in Figure 3.9, 3.10, 3.11, and 3.12. This approach of rank ordering and selecting an exceedance probability for bridge sustainability is unique to this project.

Figure 3.9 shows the amount of CO₂ produced by bridges in Colorado based on the sample of 36 bridges. From the plot, it can be understood that at least 20% of the bridges in Colorado produced more than 4,000 tons of CO₂ solely from the essential structural materials used in them, with the minimum amount of CO₂ emission from the materials being 703 tons.

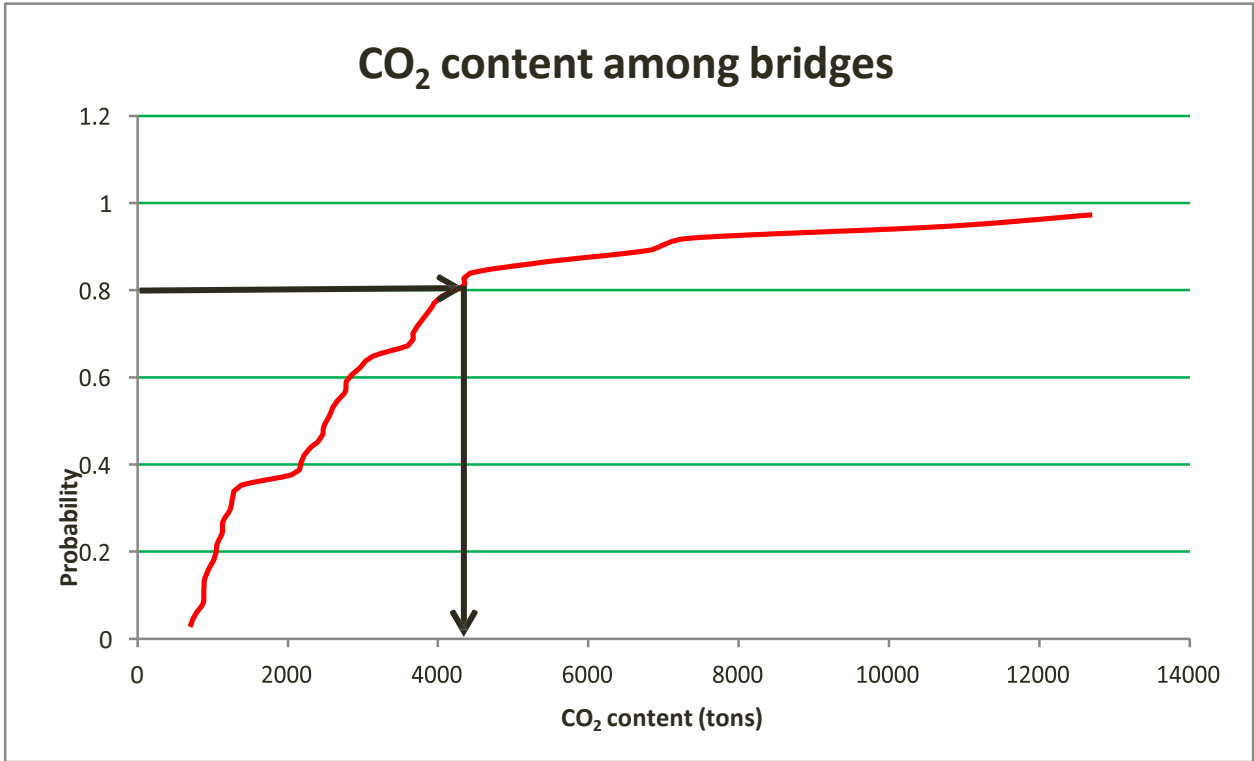


Figure 3.9 CO₂ content among bridges in Colorado

Figure 3.10 shows the cumulative distribution function of CO2 content in tons per square feet area of the deck. It suggests the amount of CO2 emission is in direct correlation with the deck area. By using the probability scale on the y axis, it is possible to derive the probability of achieving the status of sustainability, which is determined by using the ranking system discussed later.

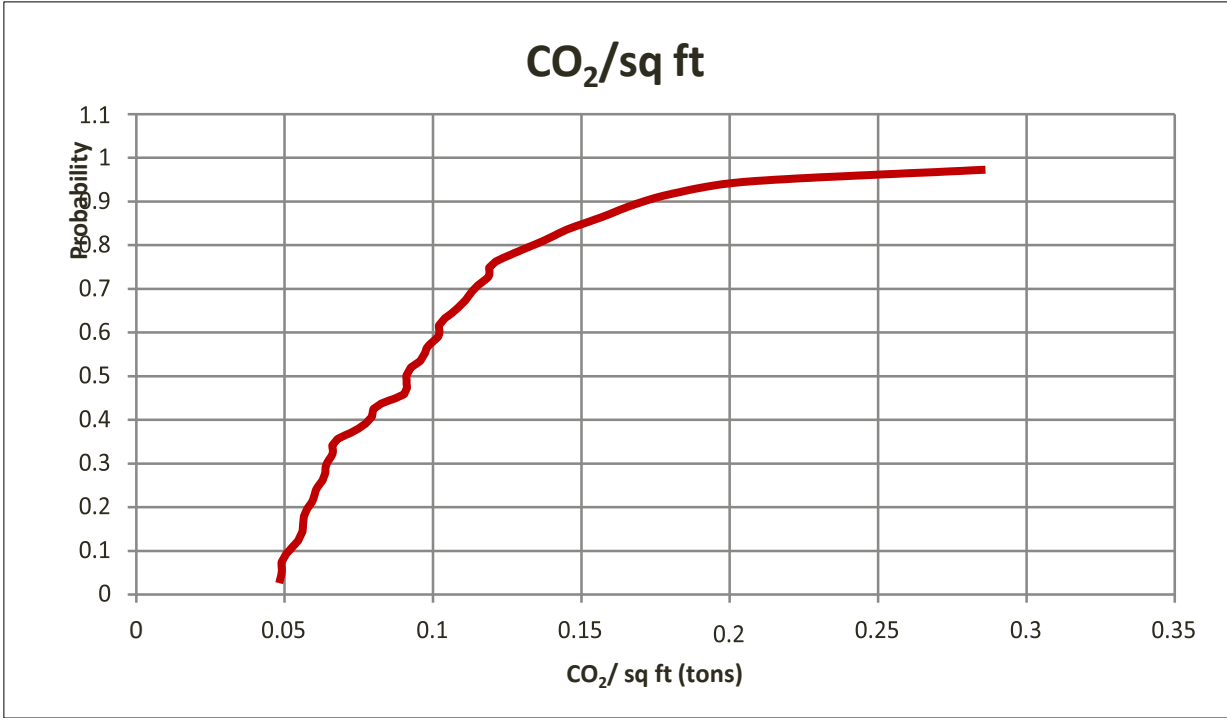


Figure 3.10 CO2 content per square feet area of deck

Figure 3.11 shows the amount of CO₂ content in tons present per lane of any given bridge.

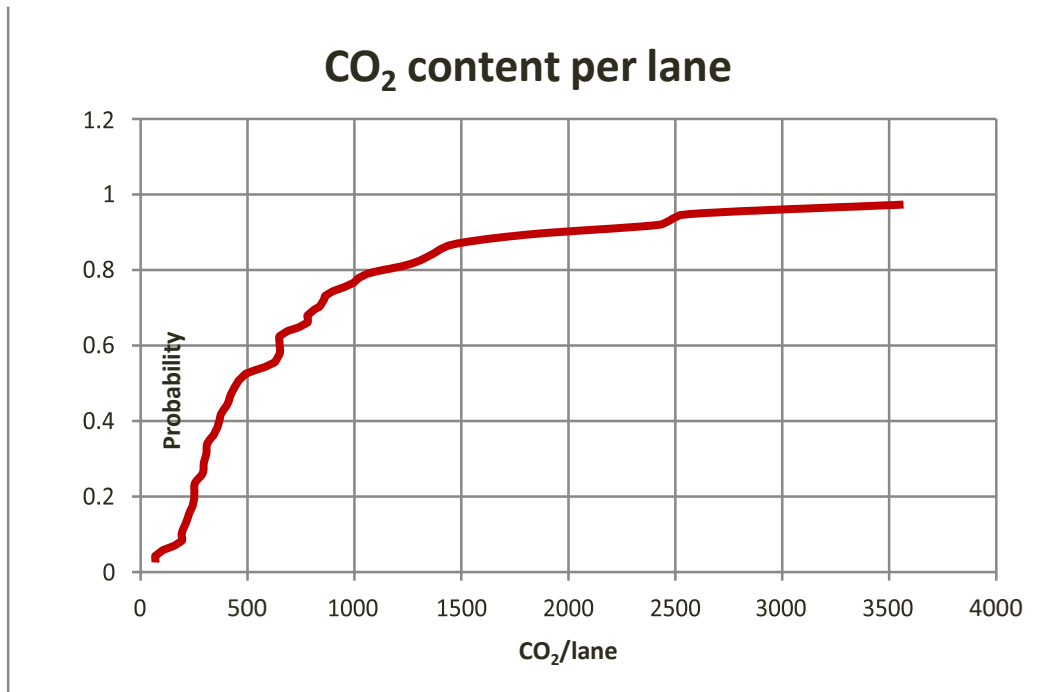


Figure 3.11 CO₂ content per (12-foot wide) lane

Figure 3.12 shows the CO₂ content in tons per unit width of the bridge. For this plot, areas occupied by a one-foot strip of bridge deck spanning the total width of lanes were considered. This type of computational approach was then used to divide the total amount of CO₂ contribution by the bridge, which was repeated for the bridges in the sample size. Graphs such as this also correlate to the CO₂ content per lane of a bridge, suggesting wider bridges contribute to higher CO₂ emissions, which is obvious since they are larger and require more construction materials.

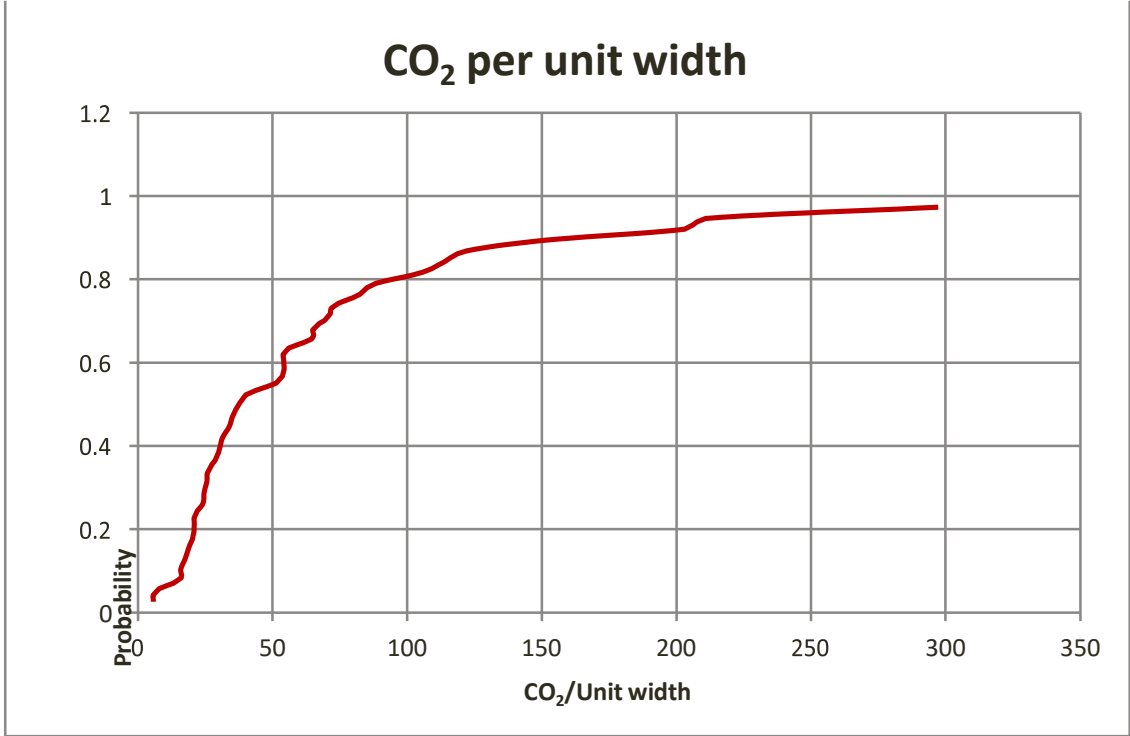


Figure 3.12 CO₂ content per unit width (1 feet) of bridge

3.4 Discussion

General/universal sustainable rating systems exist because they provide guidance for designing and constructing structures of a sustainable nature. By having such rating systems, it also becomes easier for its stakeholders to save time and money by targeting and turning their design and construction process toward the attributes deemed beneficial. Evidence indicates that the sustainable buildings attract higher rents than conventional buildings and also enjoy higher rates of rental growth (CBRE, 2009). Improved marketability for sustainable buildings is currently the main competitive advantage to reduce vacancy durations and, hence, income losses (McKee, 1998). Such advances greatly help motivate suppliers to making environmentally friendly materials to satisfy the rating systems and possibly motivate public entities, such as CDOT, to invest in eco-friendly infrastructure projects.

The study started out with the compilation of a number of available green building rating systems and analyzing their potential for application to bridges. Criteria in the rating systems that could be applied to bridges were grouped under general categories of sustainability in materials, energy, site selection, air quality, and water usage. After grouping the criteria, one of the criteria, material sustainability in terms of carbon footprint was then used as a surrogate for general sustainability and illustrated in Section 3. After making assumptions described in Section 3.1 and under the procedure described in Section 3.2, results were obtained as shown in Section 3.3.

Using results from the analyses, and with the primary objective of developing a preliminary rating system for quantifying sustainability in bridges, a simple rating system was formulated. It developed with the idea of eventually extending the concept of sustainability to more than material usage in a bridge and to provide a general guideline on how to achieve further quantification of sustainability in bridges. A breakdown of the proposed rating system is shown in Table 3.8.

Table 3.8 Ranking system for CO₂ content per square feet

Position on CDF	Corresponding ranking	CO₂/sq ft (tons)
$0 \geq y \geq 0.2$	Superior	0-0.143
$0.2 > y \geq 0.5$	Excellent	0.143-0.164
$0.5 > y \geq 0.8$	Acceptable	0.164-0.217
$0.8 > y \geq 0.9$	Poor	0.217-0.291
$0.9 > y \geq 1.0$	Unacceptable	0.291-0.496

The above mentioned rating system in Table 3.8 is also shown in figure form in Figure 3.13. CO₂ per square foot was chosen for applying the ranking system since the area of the deck is directly proportional to the number of lanes as well as area per unit width of the bridge. The bridges subjected to the analysis only comply with the assumptions stated in sections 3.1 and 3.2. While the ranking system outlined in Table 3.8 is somewhat arbitrary, it is not without logic.

The divisions in the ranking boundaries generally align with changes in the slope of the CDF curve. For example, the ranking system effectively states that bridges matching those in the lower 20% be will deemed superior, while bridges matching those in the upper 20% will be poor or unacceptable. Figure 3.13 shows the division in the empirical CDF for clarity.

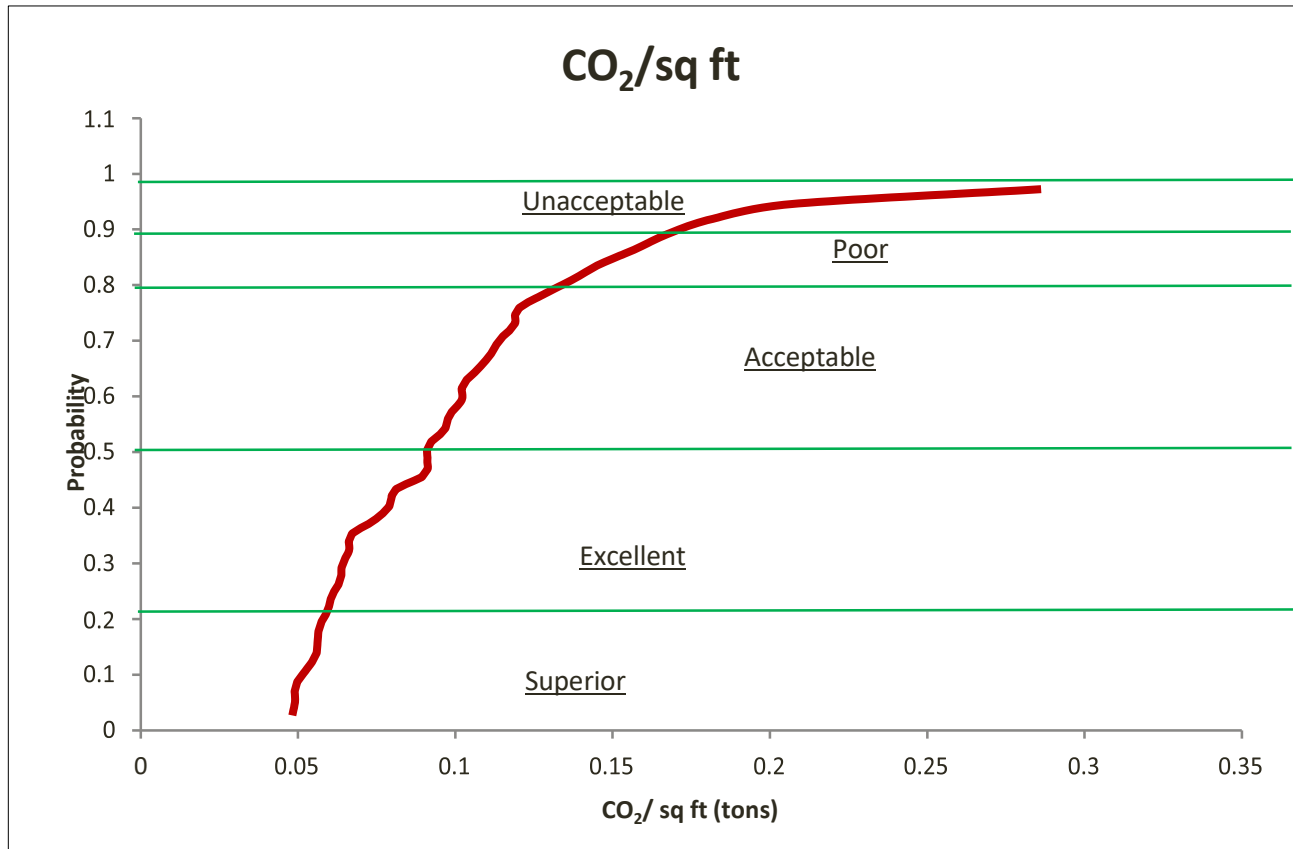


Figure 3.13 Ranking system for CO₂ content per square feet of deck area

4. SUMMARY, CONCLUSIONS AND RECOMMENDATIONS

The objective of this study was to develop a preliminary bridge sustainability rating system for trunk line bridges in the Mountain Plains region of the United States. Initially, a number of popular green building rating systems identified were assessed. They were analyzed to identify common characteristics among them in order to understand the most important attributes for sustainability in buildings, and they were further refined for their applicability to bridges. After identifying certain criteria, one criterion, material sustainability in terms of carbon footprint, was selected as a surrogate to develop a ranking system for sustainability of bridges.

Material sustainability was measured based on the equivalent amount of CO₂ emitted by the main materials (concrete and steel) used in the bridge.

A sample of 36 bridges was then selected based on a set of criteria and analyzed for its equivalent CO₂ contribution by the main material used in the construction. Analyses for this were done using the Environmental Analyses (EA) Tool developed by Skidmore, Owings & Merrill, LLP. After calculating the equivalent CO₂ contribution of each bridge's materials, they were normalized based on CO₂/square foot area, CO₂/lane, and CO₂/unit. CO₂/square feet of deck area, as shown in Figure 3.10, was chosen for developing the ranking system as described in Table 3.10 and Figure 3.13.

Using basic rank-ordering for the CO₂ emissions per square foot of bridge deck allowed a simple statistical division to be made for five different sustainability ratings: superior, excellent, acceptable, poor, and unacceptable. Each rating corresponds to a percentile within the 36 bridge populations used in the analysis.

From analyzing the ranking of bridges, it was found that prestressed bridges have the least amount of CO₂/square foot compared with steel bridges for this simplified approach. Among bridges ranked superior to excellent, 66.7% were prestressed bridges and 33.3% were steel bridges. Similarly, among bridges ranked from acceptable to poor, prestressed bridges comprised 14.3% of the sample size and the remaining 85.7% were steel bridges.

One of the major areas where the study could be improved in future analyses is in the sample size of the bridges considered, as well as in incorporating direct and indirect GHG emissions from the construction processes and end-of-life demolition. With the increase in size of the sample from 36, the ranking system developed can offer more credibility in awarding a specific bridge with its ranking. Similar to increasing the sample size, the number of materials considered in predicting CO₂ contribution of the bridge should also be increased. Since only concrete and steel are considered in this study, it should be expanded to include formwork (for cast-in-place concrete), asphalt pavement, sidewalk, architectural components, railings, street lamps, etc.

Furthermore, any future analysis could be expanded to include different direct and indirect processes essential for the construction and proper functioning of bridges and could also be an important factor in GHG emissions. Doing so can give precise results and, hence, help to decide if the bridge is sustainable not only during construction but also during its operation.

The results of this study are preliminary and not intended to be used for applications related to design selection.

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APPENDIX A

BRIDGE KEY	LENGTH (m)	MAX SPAN	MAIN SPANS	DECK WIDTH (m)	FEATURE INTERCEPTED	FACILITY	YEAR BUILT
E-17-QN	95.77	48.80	2	14.72	I 25 ML	RAMP TO US 36 WB	1998
F-16-WO	122.77	47.55	3	19.20	WADSWORTH BLVD	RTD LIGHT RAIL	2010
F-16-XG	237.35	85.83	1	10.15	US 6 AND FRONTAGE RD	RTD LIGHT RAIL	2010
F-16-XE	466.80	82.30	3	6.10	US6 AND INDIANA	RTD LIGHT RAIL	2010
E-17-RC	101.35	58.34	2	32.92	I 25 ML	SH 53 ML	1992
E-17-NZ	166.45	68.98	3	21.34	I 25 ML R	RAMP TO I 76 WBND	1991
E-17-QS,QT	101.29	54.86	2	17.98	I 270 ML	I76 ML EBND & WBND	2003
E-17-QM	256.44	57.61	5	17.70	I 25 ML, RAMPS	I 270 WBND ML	1998
E-17-SW	599.54	85.34	8	14.33	I 70 ML	I 225 ML SBND	1994
E-17-OQ	121.01	46.33	3	41.15	28TH & 30TH AVE,SAND CRK	I 225 ML	1990
E-17-OO	211.84	51.41	5	31.39	I 25 ML	US 224 ML	1991
E-17-OC,OD	172.21	70.71	3	12.50	I 25 ML	I 76 ML EBND & WBND	1990
D-20-AT	139.90	36.60	4	9.30	I 76 ML R	RAMP TO I 76 EBND	1993
E-16-MR,MS	697.53	71.93	13	17.98	I 25 ML & RAMPS	I 70 ML EBND & WBND	1991
E-16-ND	589.00	64.01	11	10.97	I 25 ML, I 70 ML,RAMPS R	RMP I70EB TO I25NB	1992
E-16-OO	85.50	43.21	2	25.15	I 25 ML	20TH STREET	1994
E-16-PJ	167.18	66.45	3	11.28	RAMP TO I 70 WBND R	RAMP TO I 70 WBND	1993
E-17-ABJ	245.55	67.43	4	9.14	NWP	Ramp B	2003
E-16-NJ	136.55	47.98	3	54.44	BNSF RR, UP RR	I 70 ML	1992
G-17-CS	155.75	55.17	4	5.82	I25 ML	UPRR	2005
G-17-DA	171.45	67.06	3	12.50	I25 ML & PLUM CREEK	5TH ST.	2001
E-17-PC	70.41	50.29	3	10.67	RAMP TO I 70 WBND R	RAMP TO I 70 EBND	1993

BRIDGE KEY	LENGTH (m)	MAX SPAN	MAIN SPANS	DECK WIDTH (m)	FEATURE INTERCEPTED	FACILITY	YEAR BUILT
B-16-GK	69.53	17.01	4	34.44	CACHE LA POUDRE RIVER	US 287 ML	1995
B-17-DS	98.48	57.99	3	33.60	I 25 ML	OLD SH 68 ML	1999
C-17-FO	73.36	36.42	2	18.17	US 34 BYPASS	SH 257 ML	1999
C-17-FP	80.53	41.00	2	18.17	US 34 BYPASS	SH 257 ML	1999
C-20-AS	160.17	31.39	5	11.13	S. PLATTE RIVER OVERFLOW	SH 39 ML	1996
C-20-AT	96.16	31.39	3	11.13	SOUTH PLATTE RIVER	SH 39 ML	1996
C-20-Q	151.33	37.79	4	11.28	SOUTH PLATTE RIVER	SH 144 ML	2001
C-21-BL	187.45	26.52	7	11.13	SOUTH PLATTE RIVER	SH 144 ML	1996
D-20-AS	118.45	23.77	5	12.34	KIOWA CREEK	I 76 ML EBND	1993
C-21-BM	106.70	52.70	2	14.17	I 76 ML R	RAMP TO I 76	1995
D-17-DJ	73.61	35.69	3	18.72	SH 119 ML	I 25 ML SBND	1998
E-16-QU	99.00	48.86	2	17.47	US 36 ML	88TH ST.	2000
D-15-BO	95.71	47.33	2	14.94	US 36 SPUR/BASELINE RD	US 36 WBND SPUR	1993
D-20-AR	118.42	23.70	5	12.32	KIOWA CREEK	I 76 ML WBND	1993

APPENDIX B

Table B1 Equivalent CO₂ content in 1kg of cement

For 1.0 kg of cement:	Emission:	Unit	Factor	Emission	Unit
Embodied carbon dioxide	0.92703	kg	1	0.92703	kg CO ₂ e
Other GHG's:					
Dinitrogen monoxide	0	kg	310	0	kg CO ₂ e
Methane	0.00004	kg	21	0.00083	kg CO ₂ e
Methane, HCC-30	0	kg	9	0	kg CO ₂ e
Nitrogen oxides	0.002503	kg	0	0	kg CO ₂ e
Nonmethane VOCs	0.00005	kg	0	0	kg CO ₂ e
Carbon monoxide	0.001105	kg	0	0	kg CO ₂ e
Total Equivalent Embodied Carbon dioxide:				0.92786	kg CO ₂ e

The emissions data for the manufacturing of sand and aggregates are determined by obtaining the required energy for the manufacture of 1 kg of the substance (in joules) and then multiplying this value by the known emissions associated with the production of 1 mega-joule of energy, assuming average contributions from various sources for the production of that energy. The energy required for the manufacturing of sand and aggregates are given in the PCA Report (PCA, 2007) as 23.19 kj and 35.44 kj, respectively. The emission value associated with 1 MJ of energy is given in Table 3.3 of this report. (SOM, 2013). Fly ash and silica manufacturing does not require any energy since they are the byproducts of other processes and require no additional processing to be used in concrete other than its transportation. Slag manufacturing requires energy to be granulated, dewatered, crushed, ground, and stored before adding to concrete. Therefore, the upstream energy is taken equal to 0.72 MJ/1kg of slag given by the PCA report (SOM, 2013 & PCA, 2007).

Additionally, the distance traveled by silica fume, fly ash, and slag contributes to the equivalent CO₂ content. Such emissions are tabulated in Table B2 and organized based on the modes of transportation (PCA, 2007). Each mode applies to a corresponding fraction of the unit of material considered and emissions from each mode needed to be summed; refer to the transportation emissions section of this report (Table 3.7) for data for 1 ton*km unit transport by each mode (SOM, 2013).

Table B2 Equivalent CO₂ emission in

Material	Truck		Rail		Barge	
	Fraction	km	Fraction	km	Fraction	km
Fly ash, silica fume, slag	0.951	146	0.039	146	0.01	702

Table B3 Equivalent CO₂ content for different concrete strengths

Mixture	Low-strength Type Mix (kg)			Average-strength Type Mix (kg)			High-strength Type Mix (kg)		
	0% Fly Ash and Slag	5% Fly Ash, 5% Slag	10% Fly Ash, 10% Slag	0% Fly Ash and Slag	5% Fly Ash, 5% Slag	10% Fly Ash, 10% Slag	0% Fly Ash and Slag	5% Fly Ash, 5% Slag	10% Fly Ash, 10% Slag
Cement	0.093	0.084	0.074	0.132	0.119	0.106	0.2	0.18	0.16
Sand	0.286	0.286	0.286	0.273	0.273	0.273	0.25	0.25	0.25
Coarse Aggregate	0.571	0.571	0.571	0.545	0.545	0.545	0.5	0.5	0.5
Water	0.013	0.013	0.013	0.025	0.025	0.025	0.013	0.013	0.013
Fly Ash	0	0.005	0.009	0	0.007	0.013	0	0.01	0.02
Slag	0	0.005	0.009	0	0.007	0.013	0	0.01	0.02
TOTAL kg:	1	1	1	1	1	1	1	1	1
TOTAL kg CO ₂ :	0.092	0.084	0.076	0.128	0.116	0.105	0.19	0.173	0.156

Table B4 Equivalent CO₂ content for each component in concrete

Substance (1 kg)	kg CO₂ (equivalent)
Cement	0.928
Sand	0.005
Coarse Aggregate	0.007
Fly Ash or Silica Fume	0.011
Slag	0.151